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# Performance Analysis of Irrigation Channels with Distributed Control

Yuping Li and Bart De Schutter

**Abstract**—For a string of pools with distant-downstream control, the internal time-delay for water transport from upstream to downstream not only limits the local control performance of regulating water-levels at setpoints and rejecting offtake disturbances in each pool, but also impacts the global performance of managing the water-level error propagation and attenuating the amplification of control actions in the upstream direction. A distributed control scheme which inherits the interconnection structure of the plant is studied. It is shown that the decoupling terms in the controller helps to improve global closed-loop performance by decreasing the low-frequency gain of the closed-loop coupling. Moreover, they compensate for the influence of the time-delay by imposing extra phase lead-lag compensation in the mid-frequency range on the closed-loop coupling function.

## I. INTRODUCTION

Water is becoming a scarce resource all over the world. Irrigation accounts for 70% of water usage [1]. Fig. 1 shows the topview of a typical irrigation network. Water is drawn

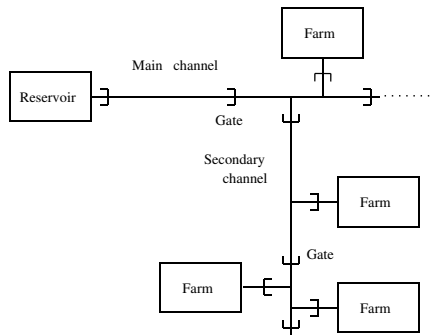


Fig. 1. Topview of an irrigation network

from the reservoir and distributed through the main channel and many secondary channels to farms. Along the channels, mechanical gates are installed to regulate the flow, as shown in Fig. 2. A stretch of water between two neighbouring gates is called a pool. An irrigation network is largely gravity-fed (i.e. there is no pumping); to satisfy water-demands from farms and to decrease water wastage, the water-levels in the pools should be regulated to certain setpoints. Since most farms sit at the downstream ends of pools, it is more important to control downstream water-levels. To avoid the excessive communication load for large-scale system, decentralised control is preferred to centralised control. In practice,

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Fig. 2. An irrigation channel (Source: Rubicon Systems Australia Pty. Ltd)

a distant-downstream control structure (i.e. use upstream gate to control downstream water-level of a pool) is implemented for good management of water service and water distribution efficiency [2]. Further, an irrigation channel is a system presenting strong interactions between pools, i.e. the flow into a pool is equivalent to the flow out of the neighbouring upstream pool. When offtakes occur at downstream pool, one could see amplification of the control action (e.g. flow over upstream gates) and water-level error propagation towards upstream, see [3], [9]. Therefore, control objectives for large-scale irrigation network involve: locally, setpoints regulation, rejection of offtake disturbances, avoiding excitement of dominant waves and, globally, management of the water-level error propagation and attenuation of the amplification of control action in the upstream direction. As shown in [9], there exists a tradeoff between the local and the global control performance. To cope with such a tradeoff, a distributed control scheme that inherits the interconnecting structure of the plant is suggested in [3], [4]. Such a distributed control scheme presents performance advantage over decentralised feedback with feedforward control [5].

In fact, one big issue in control design for an irrigation network comes from the time-delay in each pool, i.e. the time for transporting water from the upstream gate to the downstream gate. In this paper, the impact of the internal time-delays on the local and global control performance is analysed. Further, we discuss how the distributed control scheme compensates for such impact. Although the paper focuses on irrigation networks, the discussion can be extended to many practical networks that involve internal time-delay. The paper is organised as follows. Section II briefly introduces modelling of an irrigation channel and designing of the distributed controller. In Section III, discussions are made on how the distributed control scheme manages the water-level error propagation and attenuates the amplification

of control actions in the upstream direction. Section IV summarises the paper.

## II. MODELLING OF A CHANNEL AND DESIGNING OF DISTRIBUTED CONTROLLER

Fig. 3 shows an irrigation channel with a special structured distributed control, i.e. the information flow is uni-directional: from controller  $K_{i+1}$  to controller  $K_i$ . When water offtakes occur in a pool, such an interconnection structure confines the water-level error propagation and amplification of control action in the upstream pools. Hence, such a control scheme avoids the requirement of water storage at the downstream end of the channel.

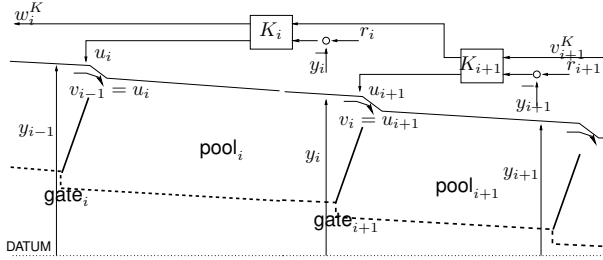


Fig. 3. Distributed control of an open water channel

### A. Plant model

A simple model of the water-level in pool<sub>*i*</sub> can be obtained by conservation of mass [3], [6]:

$$\alpha_i \dot{y}_i(t) = u_i(t - \tau_i) - v_i(t) - d_i(t),$$

where  $u_i$  is the flow over the upstream gate,  $v_i$  the flow over the downstream gate,  $d_i$  models the offtake load-disturbances from pool<sub>*i*</sub>;  $\tau_i$  is the transport delay of water from upstream gate to downstream gate of the pool, and  $\alpha_i$  a measure of the pool surface area. Note the interconnection  $v_i = u_{i+1}$ , i.e. the flow out from pool<sub>*i*</sub> equals the flow into pool<sub>*i+1*</sub>. Taking Laplace transform, yields

$$P_i : y_i(s) = \frac{1}{s\alpha_i} (e^{-s\tau_i} u_i - v_i - d_i)(s). \quad (1)$$

### B. Designing of the distributed controller

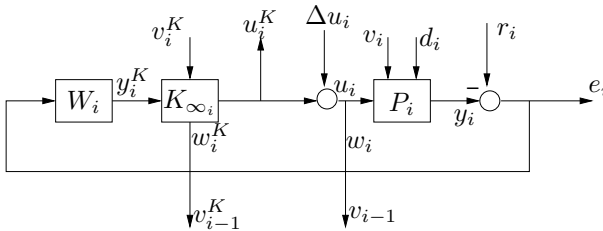


Fig. 4. Localised portion of distributed controller design

Fig. 4 shows a localised portion of a channel under distributed distant-downstream control, where  $P_i$  is the nominal model (1) for pool<sub>*i*</sub>, and  $K_i$  in Fig. 3 is split into a loop-shaping weight  $W_i$  and a compensator  $K_{\infty i}$  (with  $y_i^K$  and  $u_i^K$ , input from and output to the shaped plant,

respectively). Note the constraint on the interconnection between controllers  $v_i^K = w_{i+1}^K$ . Designing of the distributed controller consists of the following three steps, which are consistent with the well-known  $\mathcal{H}_\infty$  loop-shaping approach [11].

- 1) Design  $W_i$  to shape  $P_i$  based on local performance. Typical offtakes  $d_i$  are step disturbances; based on the internal model principle [7], a simple selection could be  $W_i = \frac{\kappa_i}{s}$  for zero steady-state water-level error. For robust stability,  $\kappa_i$  is selected such that the local crossover frequency  $\omega_{c_i} \leq 1/\tau_i$  (see [8]). Denote  $z_i := (e_i, u_i^K)^T$  and  $n_i := (r_i, \Delta u_i, d_i)^T$ , with  $r_i$  the water-level setpoint and  $\Delta u_i$  modelling additional uncertainty in flow over gate<sub>*i*</sub>. For a channel of  $N$  pools, Let  $G_s := (G_{s_1}, \dots, G_{s_N})$  denote the interconnection of the shaped plant

$$G_{s_i} := \begin{pmatrix} v_i \\ n_i^K \\ u_i^K \end{pmatrix} \mapsto \begin{pmatrix} w_i \\ z_i^K \\ u_i^K \end{pmatrix} = \begin{bmatrix} 0 & (0 \ 1 \ 0) & 1 \\ \left(\frac{1}{s\alpha_i}\right) & \begin{pmatrix} 1 & e^{-s\tau_i} & 1 \\ 0 & -s\alpha_i & s\alpha_i \end{pmatrix} & \begin{pmatrix} e^{-s\tau_i} \\ 1 \\ -s\alpha_i \end{pmatrix} \\ \frac{W_i}{s\alpha_i} & \begin{pmatrix} W_i & e^{-s\tau_i} W_i \\ -s\alpha_i & s\alpha_i \end{pmatrix} & \begin{pmatrix} e^{-s\tau_i} W_i \\ -s\alpha_i \end{pmatrix} \end{bmatrix}$$

with  $v_i = w_{i+1}$  and boundary condition  $v_N = 0$ . Note that such a boundary condition is possible with distant-downstream control.

- 2) Synthesise  $K_{\infty i}$  to cope with the tradeoff between local performance and closed-loop coupling.<sup>1</sup> Let  $K_\infty := (K_{\infty_1}, \dots, K_{\infty_N})$  denote the interconnection of

$$K_{\infty_i} := \begin{pmatrix} v_i^K \\ y_i^K \end{pmatrix} \mapsto \begin{pmatrix} w_i^K \\ u_i^K \end{pmatrix}$$

with  $v_i^K = w_{i+1}^K$  and boundary condition  $v_N^K = 0$ ; and let  $H(G_s, K_\infty)$  denote the closed-loop transfer function from  $(n_1, \dots, n_N)^T$  to  $(z_1, \dots, z_N)^T$ . The synthesis problem is formulated as

$$\begin{aligned} & \min_{K_\infty \in \mathcal{K}_{\text{syn}}} \gamma \\ & \text{subject to} \\ & \|H(G_s, K_\infty)\|_\infty < \gamma \end{aligned} \quad (2)$$

where  $\mathcal{K}_{\text{syn}}$  represents the set of stabilising  $K_\infty$ 's. Note that we use  $\|\cdot\|_\infty$  to denote the  $\mathcal{H}_\infty$  norm of a transfer function. Such a structured optimisation problem can be solved by employing the technique in [10], see [4].

- 3) The final distributed controller is then given by

$$K_i := \begin{pmatrix} v_i^K \\ e_i \end{pmatrix} \mapsto \begin{pmatrix} w_i^K \\ u_i^K \end{pmatrix} = K_{\infty_i} \begin{bmatrix} 1 & 0 \\ 0 & W_i \end{bmatrix}.$$

## III. CLOSED-LOOP PERFORMANCE

For distant-downstream control, the internal time-delay  $\tau_i$  limits the local performance. For example, the local bandwidth limit of  $1/\tau_i$  is previously considered in the selection

<sup>1</sup>For local performance, one considers  $e_i$  to be small; while closed-loop coupling is caused by control action  $u_i$  to compensate  $e_i$ . As shown in [3], [9], for purely decentralised feedback control,  $T_{r_i \rightarrow e_i} + T_{d_i \rightarrow u_i} e^{-s\tau_i} = 1$ .

of the weight gain,  $\kappa_i$ . In this section, the influences of  $\tau_i$  on the closed-loop coupling are discussed. It is shown that such time-delays, not only make it difficult to manage the water-level error propagation, but also cause the amplification of control action, in the upstream direction. Further, analysis is made on how the distributed control compensates for such influences.

#### A. The impact of $\tau_i$ on global closed-loop performance

From (1), for a channel of  $N$  pools

$$\begin{pmatrix} y_1 \\ \vdots \\ y_{N-1} \\ y_N \end{pmatrix} = \begin{bmatrix} G_1 \tilde{G}_1 & & & \\ & \ddots & \ddots & \\ & & G_{N-1} \tilde{G}_{N-1} & \\ & & & G_N \end{bmatrix} \begin{pmatrix} u_1 \\ \vdots \\ u_{N-1} \\ u_N \end{pmatrix} + \begin{bmatrix} \tilde{G}_1 & & & \\ & \ddots & & \\ & & \tilde{G}_N & \\ & & & \end{bmatrix} \begin{pmatrix} d_1 \\ \vdots \\ d_N \end{pmatrix} \quad (3)$$

where  $G_i = \frac{1}{s\alpha_i}e^{-s\tau_i}$  and  $\tilde{G}_i = -\frac{1}{s\alpha_i}$ . As previously mentioned, it is reasonable to assume  $v_N = 0$  as boundary condition for synthesis of the distributed controller under distant-downstream control. The distributed controller is represented by

$$\begin{aligned} K_1 &: u_1 = [K_1^{21} \ K_1^{22}] \begin{pmatrix} w_2^K \\ e_1 \end{pmatrix} \\ K_i &: \begin{pmatrix} w_i^K \\ u_i \end{pmatrix} = \begin{bmatrix} K_i^{11} & K_i^{12} \\ K_i^{21} & K_i^{22} \end{bmatrix} \begin{pmatrix} w_{i+1}^K \\ e_i \end{pmatrix} \\ &\text{for } i = 2, \dots, N-1 \\ K_N &: \begin{pmatrix} w_N^K \\ u_N \end{pmatrix} = \begin{bmatrix} K_N^{12} \\ K_N^{22} \end{bmatrix} e_N \end{aligned}$$

This gives the general form of the distributed controller  $K$ :

$$\begin{pmatrix} u_1 \\ \vdots \\ u_N \end{pmatrix} = \begin{bmatrix} K_{11} & \dots & K_{1N} \\ & \ddots & \vdots \\ & & K_{NN} \end{bmatrix} \begin{pmatrix} e_1 \\ \vdots \\ e_N \end{pmatrix}; \quad (4)$$

where for  $i = 1, \dots, N$ ,  $K_{ii} = K_i^{22}$ , which takes care of local performance, and the additional decoupling terms

$$\begin{aligned} K_{i,i+1} &= K_i^{21} K_{i+1}^{12}, \\ K_{ij} &= K_i^{21} \left( \prod_{k=i+1}^{j-1} K_k^{11} \right) K_j^{12} \text{ for } j > i+1. \end{aligned} \quad (5)$$

Note that  $e_i = r_i - y_i$ . Then the closed-loop relationship between water-level errors and offtake disturbances is:

$$\begin{pmatrix} e_1 \\ \vdots \\ e_N \end{pmatrix} = \begin{bmatrix} M_{11} & \dots & M_{1N} \\ & \ddots & \vdots \\ & & M_{NN} \end{bmatrix} \begin{pmatrix} d_1 \\ \vdots \\ d_N \end{pmatrix} \quad (6)$$

where for  $i = 1, \dots, N$ ,  $M_{ii} = -\tilde{G}_i (1 + G_i K_{ii})^{-1}$  and for  $j \geq i+1$

$$M_{ij} = M_{ii} \sum_{k=i+1}^j (K_{i+1,k} - K_{ik} e^{-s\tau_i}) M_{kj}. \quad (7)$$

We see that the closed-loop transfer matrix is upper-triangular, hence the multivariable system inherits the local stabilities. That is, the multivariable system is stable *if and only if* all monovaryable systems are stable. Since all the lower off-diagonal entries are null, even for model mismatch,

robustness is also inherited from local systems. A perfect decoupling is achieved if for all  $j > i$ ,

$$K_{i+1,j} - K_{ij} e^{-s\tau_i} = 0. \quad (8)$$

This requires  $K_{ij} = K_{i+1,j} e^{s\tau_i}$ , which is non-causal and hence impractical.

Next, analysis of global closed-loop performance is made on the two typical coupling properties of a (distant-downstream) controlled irrigation channel: water-level error propagation and amplification of control action. Assume only  $d_N$  occurs in the system, while  $d_i = 0$  for  $i = 1, \dots, N-1$ . Then from (6),

$$\begin{aligned} T_{e_{i+1} \rightarrow e_i} &:= M_{i,N} M_{i+1,N}^{-1} \\ &= M_{ii} (K_{i+1,i+1} - e^{-s\tau_i} K_{i,i+1}) + \\ &M_{ii} \sum_{k=i+2}^N (K_{i+1,k} - K_{ik} e^{-s\tau_i}) M_{kN} \\ &\left( M_{i+1,i+1} \sum_{k=i+2}^N (K_{i+2,k} - K_{i+1,k} e^{-s\tau_{i+1}}) M_{kN} \right)^{-1}. \end{aligned}$$

Small  $\|T_{e_{i+1} \rightarrow e_i}\|_\infty$  (e.g.  $\ll 1$ ) represents a good management of the water-level error propagation.

*Remark 1:* For the case of a string of identical pools with purely decentralised feedback control (i.e.  $K = \mathbf{diag}(K_{ii})$ ),  $T_{e_{i+1} \rightarrow e_i} = M_{ii} K_{i+1,i+1}$ . If the selected  $K_{ii}$ 's are identical for all  $i = 1, \dots, N$ , then  $\|T_{e_{i+1} \rightarrow e_i}\|_\infty > 1$  (see [3], [9]). Such a strategy, i.e. designing  $K_{ii}$  only based on local control performance, creates very strong coupling between loops (since  $\|T_{e_{i+1} \rightarrow e_i}\|_\infty$  occurs at the same frequency for all  $i$ ). Instead, to decouple the interaction between pools, one can design  $K_{ii}$ 's such that the downstream closed-loop be slower than the upstream ones.<sup>2</sup> However, it is nontrivial to cope with the tradeoff between local performance and closed-loop decoupling by simply tuning the feedback controller. In contrast, the resulted distributed controller, by taking the three steps in Section II, optimises a measure of the global performance, accounting for such a tradeoff.  $\circ$

From (4) and (6), the coupling of control actions responding to  $d_N$  is

$$T_{u_{i+1} \rightarrow u_i} := \sum_{k=i}^N K_{ik} M_{kN} \left( \sum_{k=i+1}^N K_{i+1,k} M_{kN} \right)^{-1}.$$

The following discussion shows that  $\|T_{u_{i+1} \rightarrow u_i}\|_\infty > 1$ .

For an irrigation channel with purely decentralised feedback control, i.e.  $K$  in (4) being diagonal,  $T_{u_{i+1} \rightarrow u_i} = M_{ii} K_{ii} = -\tilde{G}_i K_{ii} (1 - \tilde{G}_i K_{ii} e^{-\tau_i s})^{-1}$ . Note that  $\tilde{G}_i K_{ii}$  involves two integrators.<sup>3</sup> Applying Lemma 9.3 of [7], it is straightforward to prove  $\|T_{u_{i+1} \rightarrow u_i}\|_\infty > 1$ .

Generally, under distant-downstream control (i.e. without the constraints that  $K$  in (4) be diagonal), to compensate

<sup>2</sup>Such a scheme is similar as the one suggested in [12] for the control of a platoon of vehicles, that string instability can be avoided at the expense of successively more aggressive control laws with linearly increasing gains.

<sup>3</sup>As previously discussed, for zero steady-state water-level error, an integrator is involved in  $K_{ii}$ .

$i$	$\tau_i$	$\alpha_i$	$\psi_i$
1	6 min	10344 m <sup>2</sup>	0.349 rad/min
2	25 min	39352 m <sup>2</sup>	0.084 rad/min
3	15 min	26317 m <sup>2</sup>	0.140 rad/min

TABLE I

POOL MODEL PARAMETERS: DELAY ( $\tau_i$ ), SURFACE AREA ( $\alpha_i$ ) AND WAVE FREQUENCY ( $\psi_i$ )

the influence of the internal time-delay, the amplification of control action in the upstream direction is unavoidable. This is shown in Fig. 5. Initially, the system is at steady-state.

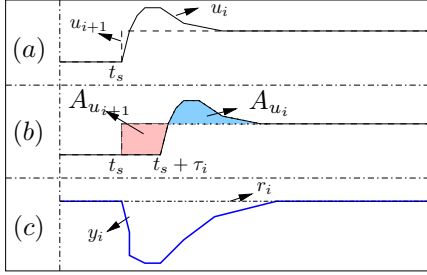


Fig. 5. Control actions for zero steady-state water-level error

At time  $t_s$ , the flow out of pool <sub>$i$</sub>  increases, see the change of  $u_{i+1}$  (the dashed line in Fig. 5(a)). To compensate for the influence of  $u_{i+1}$  on  $y_i$ , the flow into the pool,  $u_i$ , also increases (the solid line in Fig. 5(a)). However, the influence of  $u_i$  on the downstream water-level  $y_i$  will be  $\tau_i$ (min) later than that of  $u_{i+1}$  on  $y_i$  (see Fig. 5(b)). For zero steady-state error of  $y_i$  from  $r_i$  (see Fig. 5(c)), from (1),  $u_i$  should be greater than  $u_{i+1}$  for some time such that the area of  $A_{u_i}$  is equivalent to the area of  $A_{u_{i+1}}$ . Hence,  $\|T_{u_{i+1} \rightarrow u_i}\|_\infty > 1$ .

In Section III-B, the analysis focuses on the impact of the decoupling terms in the distributed controller on the closed-loop performance.

### B. The influence of $K_{ij}$ ( $j > i$ ) on closed-loop decoupling

As discussed in Section II-B, the synthesis of  $K_\infty$  copes with the tradeoff between the local performance and the decoupling of the closed-loop system. To see how the distributed controller compensates for the influence of internal time-delays, we study the time and frequency responses of a string of three pools with distributed control.

The three pools are taken from Eastern Goulburn No 12, Victoria, Australia. Table I gives the identified model parameters [13]. To shape the plant, we choose  $W_1 = \frac{87.206}{s}$ ,  $W_2 = \frac{20.8865}{s}$ ,  $W_3 = \frac{32.6255}{s}$ .<sup>4</sup> A  $\gamma = 3$  is achieved by solving the structured optimisation problem (2). The final controller is shown in Fig. 6. All the terms involve an integrator, which comes from the shaping weight. Note that  $K_{12}$  has similar phase property as  $K_{22}$ , i.e. they both involve phase-lead-lag-lag-lead compensation around the same mid-frequency range; while  $K_{13}$ ,  $K_{23}$  have similar phase property as  $K_{33}$ .

<sup>4</sup>As formerly discussed, the weight gains are chosen to set the loop-gain bandwidth just below  $1/\tau_i$  rad/min.

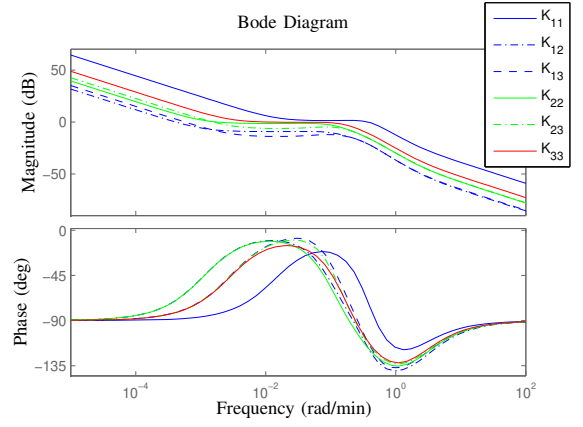


Fig. 6. The distributed controller

Fig. 7 shows the open loop-gain for pool<sub>1,2,3</sub>. High gain at low frequency is obtained, with the bandwidths 0.0408 rad/min, 0.0085 rad/min and 0.0132 rad/min respectively. Around the wave frequencies, the loop-gains are around  $-20$  dB,  $-20$  dB and  $-25$  dB respectively. This ensures no excitement of dominant waves in all the three pools.

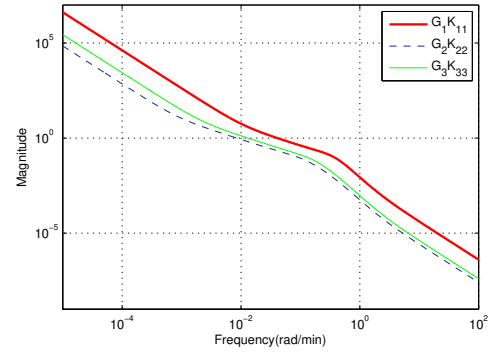


Fig. 7. Local loop-gain with the distributed controller

From (5),  $K_{12}$  and  $K_{23}$  have a similar structure, while  $K_{13}$  involves  $K_{22}^{11}$  for decoupling. The following analysis is made by checking the impact of  $K_{23}$ <sup>5</sup> and  $K_{13}$  on decoupling of the closed-loop system.

1) *Impact of  $K_{23}$* : The gains of  $T_{d_3 \rightarrow e_2}$  and  $T_{d_3 \rightarrow u_2}$ , with and without  $K_{23}$ , are given in Fig. 8. With  $K_{23}$ , a lower gain in the mid-frequency range is achieved.

Fig. 9 shows that  $K_{23}$  helps in decreasing  $|T_{e_3 \rightarrow e_2}|$  and  $|T_{u_3 \rightarrow u_2}|$  at the low and middle-frequency range, where  $d_3$  is significant. One can thus expect a good management of the water-level error propagation and attenuation of the amplification of control action with  $K_{23}$ .

The time response of the close-loop system is shown in Fig. 10 and 11. In the simulation, the water-level setpoints are set as  $r_i = 10$  m, for  $i = 1, 2, 3$ . Note that  $\tau_2$  is much bigger than  $\tau_3$ ; such a combination, i.e. a long upstream pool with a

<sup>5</sup>Similar impact of  $K_{12}$  as that of  $K_{23}$  on the closed-loop decoupling can be expected and hence the analysis is omitted here.

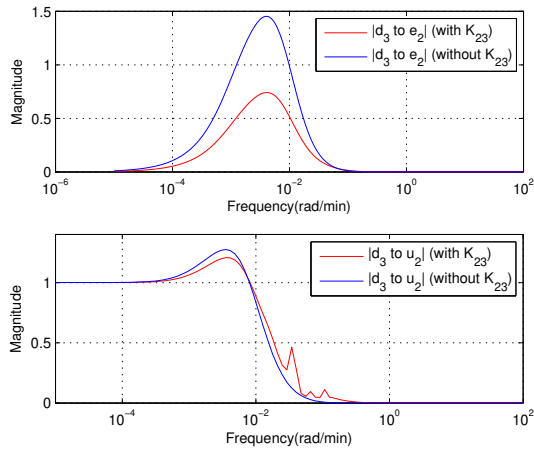


Fig. 8.  $|T_{d_3 \rightarrow e_2}|$  (top) and  $|T_{d_3 \rightarrow u_2}|$  (bottom), with and without  $K_{23}$

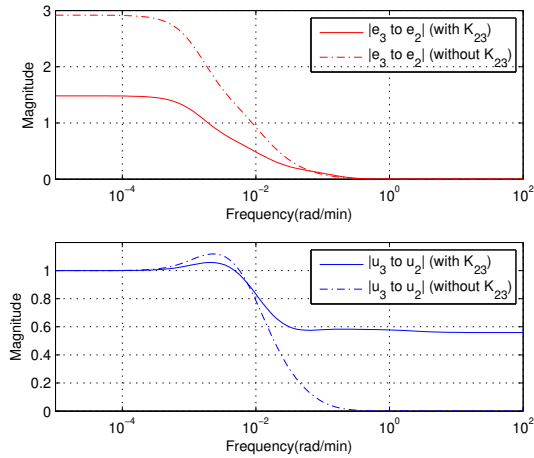


Fig. 9. Closed-loop coupling:  $|T_{e_3 \rightarrow e_2}|$  and  $|T_{u_3 \rightarrow u_2}|$

short downstream pool, is difficult for managing the tradeoff between the local water-level error and the amplification of control action.<sup>6</sup> When an offtake of 98.6 ML/day starts in

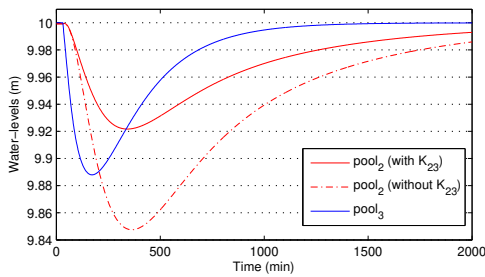


Fig. 10. Water-level error propagation: with and without  $K_{23}$

pool<sub>3</sub> at 30 min till the end of the simulation scenario, the water-level error in pool<sub>2</sub> is better managed with  $K_{23}$  operating in the system than without  $K_{23}$ . Indeed, with  $K_{23}$ ,  $\max_t |e_2(t)|$  decreases about 0.08 m (compare the red solid

<sup>6</sup>As previously discussed, to decouple the closed-loop system, one should try to make the downstream loop slower than the upstream loop.

line with the red dashed line). This is important since, as discussed in Section I, in gravity-fed irrigation networks, water-levels represent the capacity to serve water-demands at the offtake points. Fig. 11 shows the upstream control actions in pool<sub>2,3</sub> to compensate the influence of  $d_3$  on  $e_2$  and  $e_3$ .<sup>7</sup> With  $K_{23}$ ,  $u_2$  responds to the change of  $u_3$  faster than without  $K_{23}$  operating on the closed-loop. Note  $\max_t |u_2(t)|$  is smaller with  $K_{23}$ , i.e. a better attenuation of the amplification of control action is obtained.

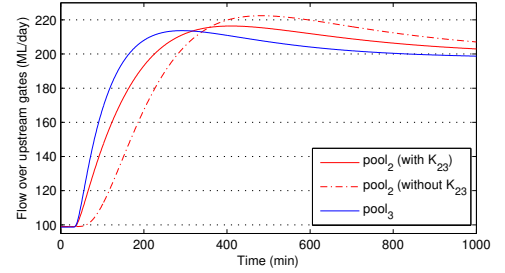


Fig. 11. Amplification of control actions: with and without  $K_{23}$

2) *Impact of  $K_{13}$* : Fig. 12 shows  $|T_{d_3 \rightarrow e_1}|$  and  $|T_{d_3 \rightarrow u_1}|$ , with and without  $K_{13}$ .<sup>8</sup> With  $K_{13}$ , a lower gain in the low and mid-frequency range is achieved, hence better decoupling of the closed-loop system can be expected. This is confirmed by the time responses shown in Fig. 13 and 14. When  $d_3$  starts at 30 min, the water-level error in pool<sub>1</sub>

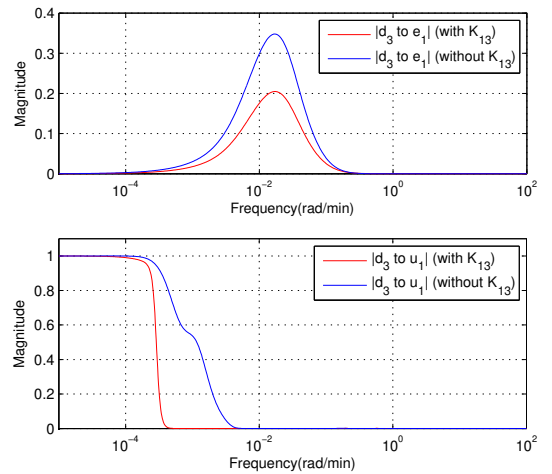


Fig. 12.  $|T_{d_3 \rightarrow e_1}|$  (top) and  $|T_{d_3 \rightarrow u_1}|$  (bottom), with and without  $K_{13}$

is smaller with  $K_{13}$  (see the green solid line in Fig. 13) operating in the system than without  $K_{13}$  (the green dash-dot line). Fig. 14 shows the change of control actions in pool<sub>1,2,3</sub> in response to  $d_3$ . We see that with  $K_{13}$ ,  $u_1$  reacts faster to the change in  $u_2$  than the case without  $K_{13}$ . Moreover,  $\|u_1\|_\infty$  is smaller with  $K_{13}$ .

<sup>7</sup>For clarity, we zoomed in to the first 1000 mins to show the changes of the control actions when  $d_3$  starts. Note we did the similar in Fig. 14.

<sup>8</sup>For the case of  $K_{13} = 0$ , it is assumed that  $K_2^{11} = 0$ , while  $K_{12}$  and  $K_{23}$  still operate on the closed-loop.

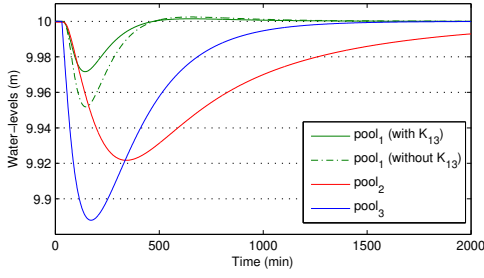


Fig. 13. Water-level error propagation: with and without  $K_{13}$

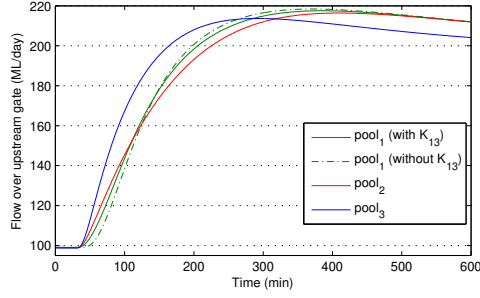


Fig. 14. Control actions: with and without  $K_{13}$

3) *Some remarks:* The closed-loop coupling term  $M_{ij}$  (see (7)) is composed of  $M_{ij}^k := M_{ii}(K_{i+1,k} - K_{ik}e^{-s\tau_i})M_{kj}$  for  $k = i + 1, \dots, j$ . Fig. 15 shows the impact of  $K_{ik}$  on  $M_{ij}^k$  in the above three-pool example. It is observed that

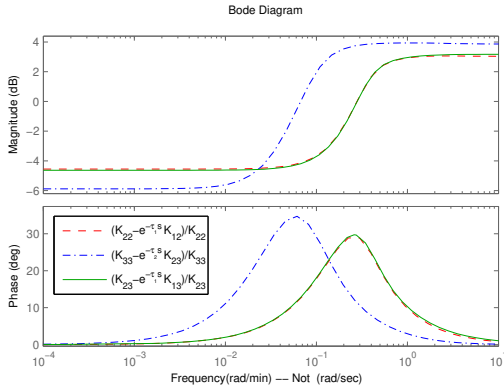


Fig. 15. The decoupling function of  $K_{ik}$  for  $k = i + 1, \dots, j$

- 1)  $K_{ik}$  decreases the gain of  $M_{ij}^k$  at low frequencies where typical offtake disturbances are significant;
- 2)  $K_{ik}$  operates on  $M_{ij}^k$  by imposing on  $M_{ii}K_{i+1,k}M_{kj}$  an additional phase lead-lag compensation around the frequency of  $1/\tau_i$ .

The first observation explains why with  $K_{ij}$  operating on the closed-loop, a better management of water-level error propagation is achieved (see Fig. 10 and 13). Although it is difficult to directly make conclusions of global performance from the second observation, time-responses of control actions (see

Fig. 11 and 14) show that with the  $K_{ij}$ 's the closed-loop predicts the influence of the internal time-delays and that the control action in response to offtake disturbance is faster than that without the  $K_{ij}$ 's.

#### IV. SUMMARY

An irrigation channel is a system presenting strong interactions between pools. This paper considers distant-downstream control of irrigation channels. It is shown that the internal time-delay for transportation of water from upstream to downstream of each pool not only limits the local performance, but also impacts the coupling between pools, i.e. the water-level error propagation and the amplification of control actions in the upstream direction. More specifically, we have discussed a distributed control that inherits the interaction structure of the plant. The controller is designed in a structured  $\mathcal{H}_\infty$  loopshaping approach. The involved optimisation problem manages the tradeoff between local and global performance. Analysis shows that the distributed controller compensates the time-delay influence by decreasing the low-frequency gain of the close-loop coupling term and imposing extra phase lead-lag compensation in the mid-frequency range on the closed-loop coupling term.

Based on the above observations of the function of the decoupling terms of the distributed controller, it is of interest in future research to investigate the involvement of similar components, e.g. phase lead-lag in decentralised feedforward compensators, in addition to the purely decentralised feedback controller, for a better global closed-loop performance.

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